

RED-ACT Report

Real-time Earthquake Damage Assessment using City-scale Time-history analysis

Oct. 30, M6.9 Zozekanissos, Greece Earthquake

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Acknowledgments and Disclaimer

The authors are grateful for the data provided by **Kandilli Observatory and Earthquake Research Institute**. This analysis is for research only. The actual damage resulting from the earthquake should be determined according to the site investigation.

Scientific background of this report can be found at: <http://www.luxinzheng.net/rr.htm>

1. Introduction to the earthquake event

At 13:51 Oct. 30 2020 (Local Time, UTC +2), an M 6.9 earthquake occurred in **Zozekanissos, Greece**. The epicenter was located at 26.82 37.89, with a depth of 10.0 km.

2. Recorded ground motions

1 ground motions near to epicenter of this earthquake were analyzed. The names and locations of the stations can be found Table 1. The maximal recorded peak ground acceleration (PGA) is **979.1** cm/s/s. The waveform and corresponding response spectra in comparison with the design spectra specified in the Chinese Code for Seismic Design of Buildings are shown in Figure 1.

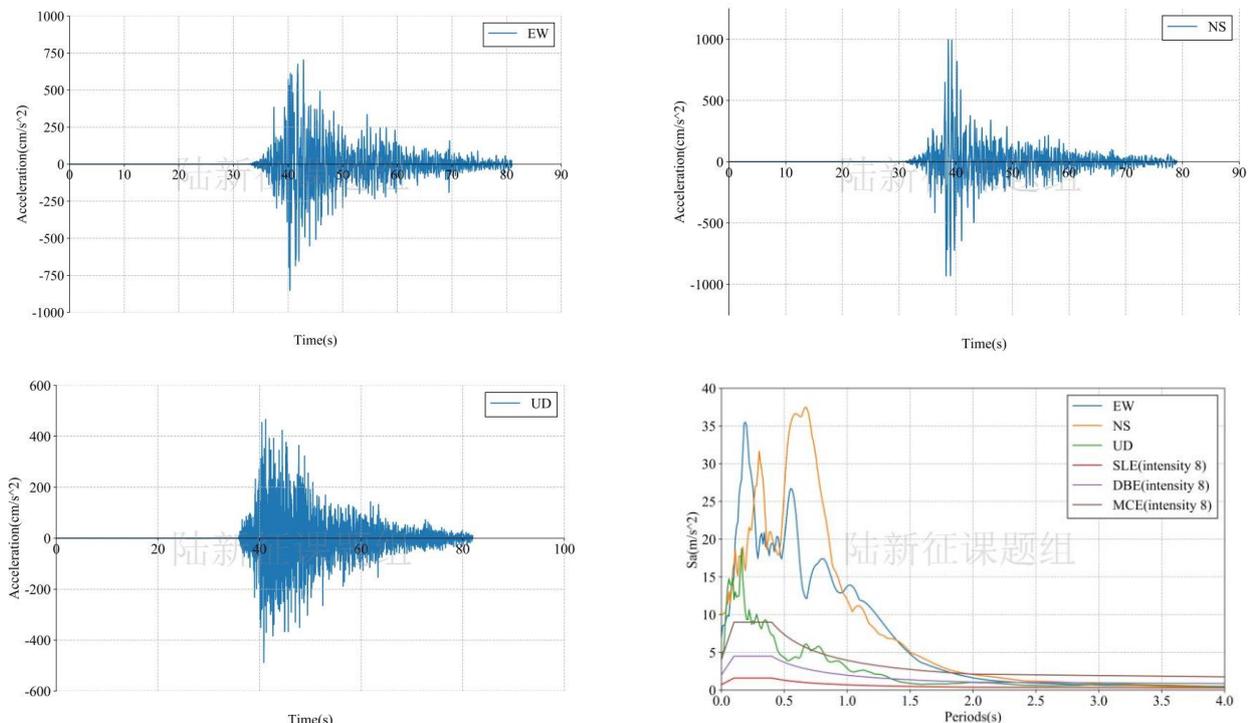


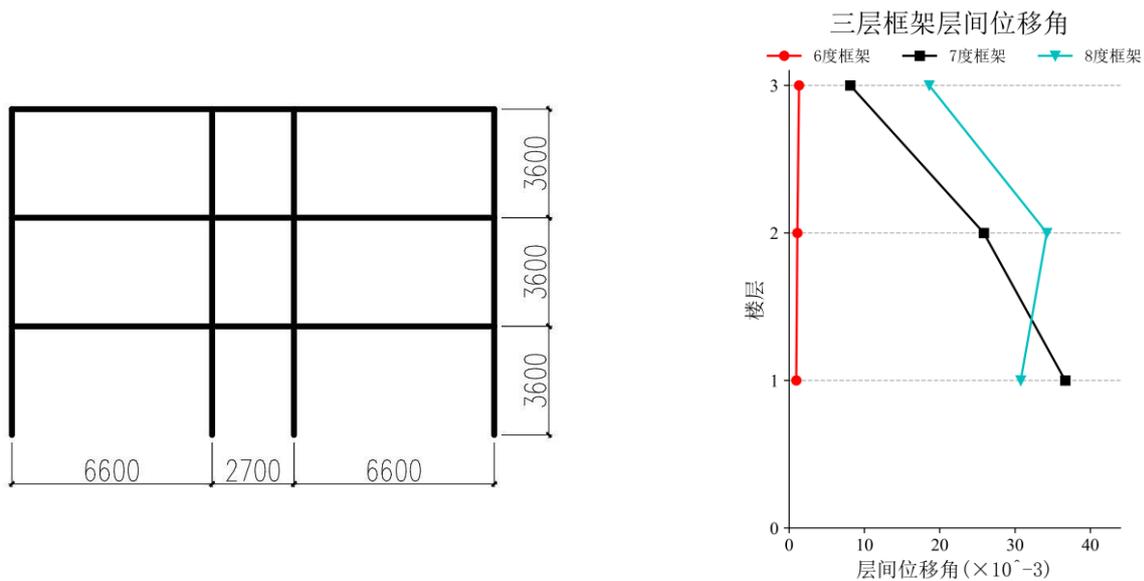
Figure 1 Waveform and response spectra of the recorded ground motions with maximal PGA

3. Seismic damage analyses for typical buildings subjected to the recorded ground motion

(1) Multi-story reinforced concrete frames

Model 1: 3-story reinforced concrete frame (Thanks Prof. Wang Qi from China Architecture Design & Research Group for providing the model)

The typical records are input into three typical 3-story reinforced concrete (RC) frames (Figure 2(a)), with seismic design intensities of 6-, 7-, and 8-degrees, respectively. The envelope of the inter-story drift ratios obtained from the nonlinear time-history analyses are shown in Figure 2(b).



(a) Elevation view (unit: mm)

(b) Envelope of the inter-story drift ratios
(6-degree design building collapsed)

Figure 2 Three typical 3-story RC frames

(2) Multi-story reinforced masonry structures

Model 1: 1-story unreinforced masonry

The typical records are input into a single story unreinforced masonry in Figure 3. The damage state of the structure is **Collapse**. (Ji X D, et al. Shaking table test of unreinforced and retrofitted brick-wood structures representative of existing rural buildings in Beijing. 2012, 11, 53-61.)



Figure 3 The shaking table test of a 1-story unreinforced brick-wood residential structure with three rooms

Model 2: 5-story simple masonry

The typical records are input into a 5-story simple masonry in Figure 4. The damage state of the structure is **Extensive damage**. (Zhu B L, et al. Seismic resistance capacity analysis of a five-story masonry test building in Shanghai)

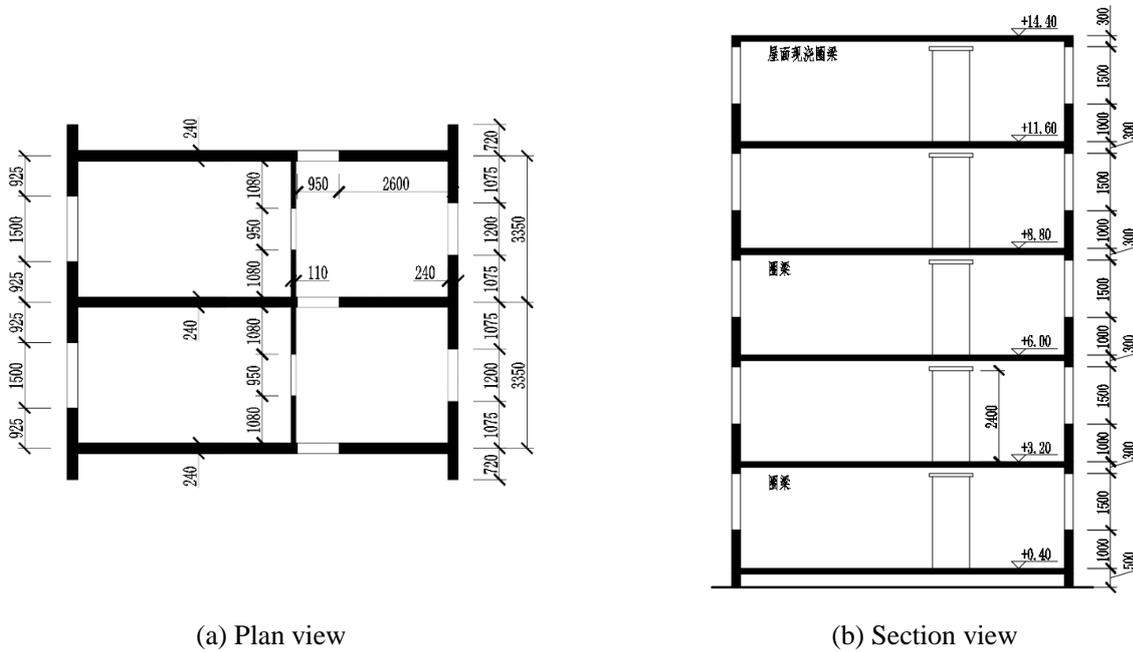


Figure 4 A 5-story simple masonry

(3) Typical bridges

Model 1: Highway bridge built in 1980s (Thanks Prof. Gu Yin from Fuzhou University)

Typical records are input into a highway bridge built in 1980s in Figure 5. The damage state of the bridge is **Extensive damage**.

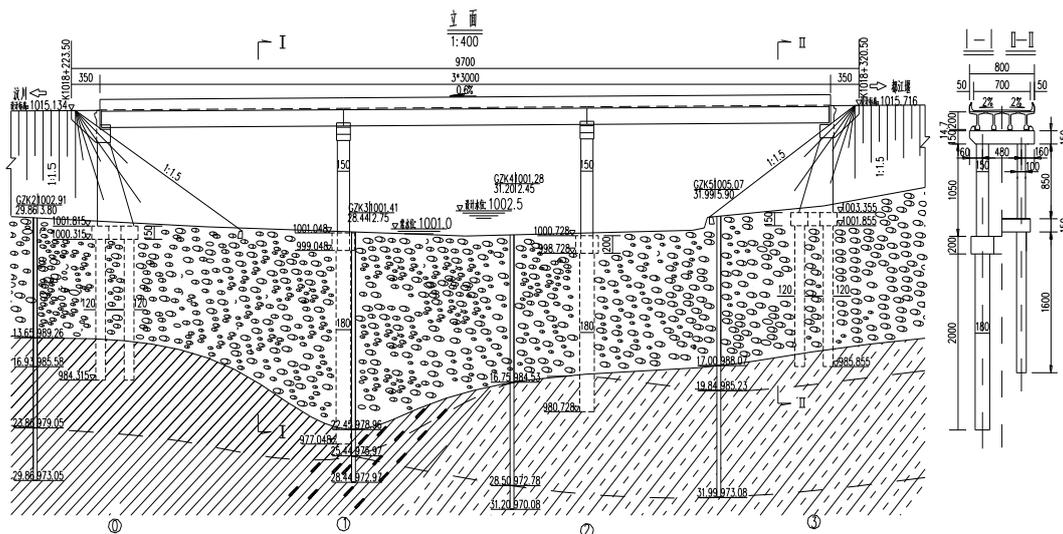


Figure 5 A highway bridge built in 1980s

Model 2: Approach bridge of a super large bridge (Thanks Prof. Gu Yin from Fuzhou University)

The typical records are input into the approach bridge of a super large bridge in Figure 6. The damage state of

the bridge is **Extensive damage**.

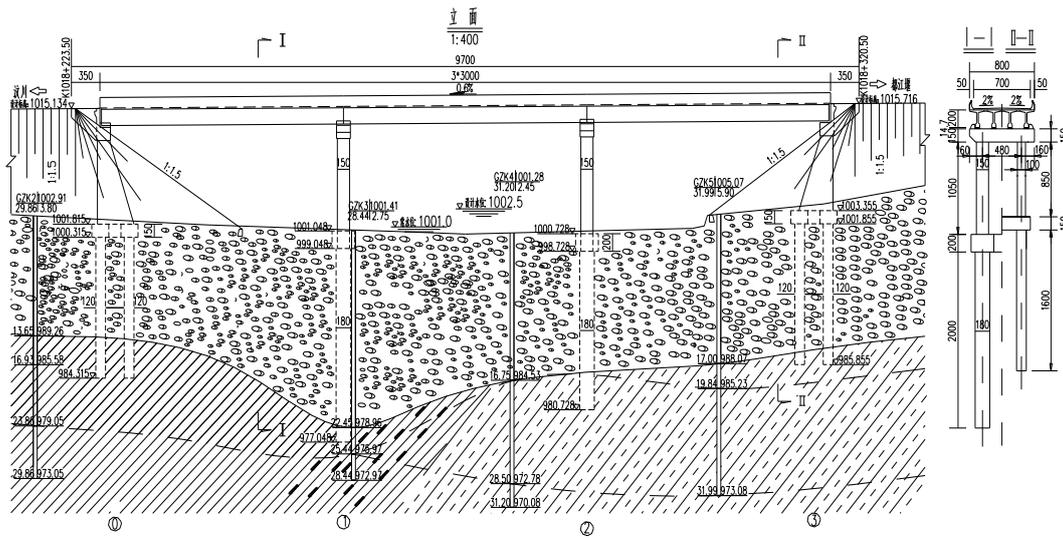


Figure 6 The approach bridge of a super large bridge

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Table 1 Names and locations of the strong motion stations

| No. | Station Name | Latitude | Longitude |
|-----|--------------|----------|-----------|
| 1 | GMLD | 38.076 | 26.875 |